Course Title: Structural Engineering X	Credit Hours 2.0	Course Code: CE 425
Time: Thour	Credit Hour: 2.0	Full Marks. 40
QUESTION 1 [8 MARKS]		[8]
Explain the hydration of four main compou	unds of cement and their effe	ects on strength
development of concrete.		
<b>QUESTION 2: [8 MARKS]</b>		[8]
Discuss the effects of Superplasticizer on fi	resh and hardened concrete.	
		[0]
QUESTION 3: 18 MARKS		[8]
Ready mix concrete (RMC) is preferably us Justify your answer with advantages and di	sed in large projects – Do ye sadvantages of RMC.	ou agree?
<b>QUESTION 4 [16 MARKS]</b>		
a. A cement with a Silica ratio of 1.50	, Alumina ratio of 1.55, Hy	draulic modulus of 2.0

- a. A cement with a Silica ratio of 1.50, Alumina ratio of 1.55, Hydraulic modulus of 2.0 and Lime saturation factor of 1.02 is selected. Determine the missing oxide percentages [8]
- b. Comment of the properties of cement in terms of strength, heat of hydration, setting time. [8]

Oxides	Content (%)
Cao	?
SiO <sub>2</sub>	?
Al <sub>2</sub> O <sub>3</sub>	7.50
Fe <sub>2</sub> O <sub>3</sub>	?
SO3	?

# University of Asia Pacific Department of Civil Engineering Mid Semester Examination Fall 2024 Program: B. Sc. Engineering (Civil)

Course Title: Environmental Engineering III	Course Code: CE 431
Time- 1 hour	Full marks: 40

### Answer all the questions (15 + 25 = 40). [Assume reasonable data if any]

1. (a) Discuss which properties (Physical/Chemical/Biological) of the following waste (8) (individually) are important to know to plan management of those wastes:

i) Alcohol, ii) Garden trimming, iii) Kitchen waste, iv) medical waste

(b) Calculate the energy content of the waste using Dulong's formula mentioned below: (7)
 Dulong's Formula:

Ash

5

32.00

Energy Content (KJ/Kg) = 338.2C+1430 (H-O/8) + 95.4SSulfur Nitrogen Moisture Carbon Hydrogen Oxygen Component 40.00 0.5 0.3 Mass (kg) 50 6 25 **Molar Mass** 

12.00

(kg/mol)

2. (a) Explain the beneficial consequences of climate change with respect to source reduction. Evaluate what kind of on-site processing of waste you can dattempt at home for decreased (4+4) burden in disposal and increased collection efficiency.

1.00

16.00

14.00

- (b) Explain your understanding of Primary and Secondary recycling. (5)
- (c) A stationary system uses a compactor with a volume of 50 m<sup>3</sup> that incurs cost @BDT (12) 4000/hour with density of waste in the compactor @500 kg/m<sup>3</sup>. On the other hand, a transfer/transport system uses a unit with a capacity of 150 m<sup>3</sup> that incurs cost @BDT 5000/hour with density of waste in the unit @200 kg/m<sup>3</sup>. The transfer system has an additional operating and unloading cost of BDT 150/ m<sup>3</sup>. Evaluate the breakeven time beyond which the economic benefit switches from one system to the other with time.

Course Title: Irrigation and Flo	od Control	Course Code: CE 461
Time: 1 hour	Credit Hour: 3.00	Full Marks: 40
	Answer all the questions	

### Answer all the questions

### **QUESTION 1 [10 MARKS]**

- a. State two considerations for ground water as a source of irrigation water [2]
- b. i) Classify the irrigation water having the following characteristics: Concentration [4+2+2] of Na, Ca and Mg are 250, 18 and 6 milligram per litre and the electric conductivity is 450 micro mhos per cm at 25°C.
  - Given are:
  - Atomic weight of sodium= 23
  - Atomic weight of calcium= 40
  - Atomic weight of magnesium= 24.3
  - ii) Identify two problems that may arise from using this water on fine textured soils.
  - iii) Provide remedies to overcome these troubles.

### **QUESTION 2 [10 MARKS]**

a. Compute the net irrigation requirement the given information. [10]  $T_{max}$ = 32.5 °C,  $T_{min}$  = 14.2 °C, Relative humidity = 30 %, Wind speed  $U_2$ = 18.5 km/day, Net radiation in equivalent evaporation = 5.5 mm/day, c = 1.0, w = 0.75, Crop coefficient = 1.2, Effective rainfall = 1.6 mm/day, Percolation = 5 mm/day, Increase in soil moisture over a day = 0.6 %, Apparent specific gravity of soil = 1.20, and, Root zone = 30 cm. [Hint: Use FAO Penman equation to calculate reference crop evapotranspiration FAO Penman equation:  $E_{t0} = c[wR_{ne} + (1 - w)f(u)(e_s - e_a)]$ , Equation for calculating vapor pressure: e(T)= 0.611 exp [ $\frac{17.27T}{T+237.3}$ ], kpa]

### **QUESTION 3 [10 MARKS]**

- a. Demonstrate the forces by which water is held in the soil. [2]
- b. Explain the importance of measuring soil moisture for irrigation and identify an [3] accurate method to measure it.

c. Calculate the depth and frequency of irrigation required for a certain crop with data [5] given below: Root zone depth = 100 cm Field capacity = 22 % Wilting point = 12 % Apparent specific gravity of soil = 1.50 Consumptive use = 2.5 cm/day Assume 50 % depletion on available moisture before application of irrigation water at field capacity.

### **QUESTION 4 [10 MARKS]**

- a. 800 m<sup>3</sup> of water is applied to a farmer's rice field of 0.6 hectares. Prior to irrigation, [6] the moisture content was reduced by 60 % of the available water between field capacity of 36 % and permanent wilting point of 15 %. Determine the water application efficiency. The root zone depth of rice is 60 cm and dry unit weight of the soil is 10.8 kN/m<sup>3</sup>. Assume any other data not given.
- b. The water depths along the length of a border strip at points 30 meters apart were [4] probed after irrigation. Their observed values are 2.0, 1.9, 1.8, 1.65 and 1.4. Compute the water distribution efficiency.

Course Title: Transportation Engineering II Time: 1 hour Credit Hour: 3.00 Course Code: CE 451 Full Marks: 60

#### Answer all the questions

### PART A

### **QUESTION 1** [12 MARKS]

- a. Differentiate between the load distribution pattern of flexible and rigid pavement with neat sketch of different layers. [5] [4]
- b. Briefly explain tie bars and dowel bars.
- c. Suppose you need to construct a cement concrete pavement on a low volume highway. Discuss the type of pavement need to be selected and justify your answer [3]

### **QUESTION 2 [8 MARKS]**

The following results were obtained by a sieve analysis. Classify the soil according to the AASHTO classification system and calculate the group index. Discuss whether this material is suitable in its natural state for use as a subbase material. [8]

Sieve No.	Percent Finer	Plasticity Test
10	92	Liquid Limit = 40
40	90	Plastic Limit = 31
100	85	
200	80	

#### Table 1: Mechanical Analysis

### PART B

### **QUESTION 3 [10 MARKS]**

a. Explain the process of obtaining bitumen as a short residue from petroleum crude oil? [7] b. Describe the significance of conducting Aggregate Crushing Value test in roadway construction. [3]

### **QUESTION 4 [30 MARKS]**

a. Determine the optimal asphalt content for a pavement designed to facilitate local travel from the village to the Sadars. The ESAL for the road is  $2 \times 10^3$ . Table 2 shows data obtained from the Marshall test. The total thickness of the mix is 67 mm. Compare the stability, flow and percent voids in total mix of the paving mixture containing the optimum asphalt content with the requirements for Marshall mix design of the pavement specified in Table 3. [25]

Table 2: Marshall Method

Asphalt	Weight	Weight	Stability	Flow	Maximum
%	of the specimen	of specimen	(16)	(111)	Gravity
	in Air (gm)	in Water (gm)			
3.0	782	625	1000	10	6.50
3.5	1050	875	800	12	7.50
4.0	860	740	945	20	8.44

b. Evaluate the suitability of your design by determining whether the parameters meet the specified limits outlined in Table 2. [5]

Course Title: Professional P	ractices and Communication	Course Code: CE 403
Time: 1 hour	Credit Hour: 2.00	Full Marks: 40

### Answer all the questions

### **QUESTION 1 [10 MARKS]**

Describe the strategies you would apply to ensure deliverables consistently meet quality standards and achieve client approval during the Implementation phase of the project. Also, explain how you would apply a comprehensive lessons-learned analysis during project closure to optimize future project outcomes.

[06+04]

[04+06]

### **QUESTION 2 [10 MARKS]**

The 'Town Center Road Project' is a multi-phase civil construction project aimed at improving traffic flow in the town center. The project's initial phase, 'Road Widening Phase 1', focused on expanding a key arterial road. This phase, funded by a substantial initial grant, has seen progress, with groundwork completed and initial paving laid. However, unforeseen utility line issues caused delays and increased material costs, leading to a budget shortfall.

The project team now needs to secure funding for the second phase, 'Road Widening Phase 2', which involves completing the road surfacing, installing sidewalks, and adding street lighting. The initial grant was intended to cover the entire road widening, but the unforeseen issues have created a funding gap.

Given the project's ongoing nature and the need to secure additional funding for the completion of the 'Town Center Road Project,' determine the most appropriate type of project proposal and provide justification for your answer by explaining the key characteristics of this proposal type and how they align with the project's current status and funding needs.

### **QUESTION 3 [10 MARKS]**

Your civil engineering firm has been selected to design and oversee the 'Downtown Redevelopment Project,' a complex urban renewal initiative. During the contract negotiation phase, the client, a city municipality, expresses concerns about several standard terms and conditions. While the budget is generally agreed upon, disagreements arise regarding the

scope of services, the allocation of risk for potential site contamination, and the municipality's desire for specific performance guarantees not included in your firm's standard contract.

The client is particularly concerned about the potential discovery of hazardous materials during excavation, which could lead to significant project delays and cost overruns. They propose shifting the majority of this risk to your firm. Additionally, they want to modify the general conditions related to project change orders and dispute resolution.

Considering the complexity of the 'Downtown Redevelopment Project' and the client's specific concerns, which *type* of contract format (e.g., Conventional, Negotiated Terms and Conditions, Multiple Contracts, Special Contracts, Client - Developed Contract) would be most applicable? Back up your answer by proper justification.

### **QUESTION 4 [10 MARKS]**

The infrastructure development company has been awarded the 'Waterline Expansion Project', a contract to extend the town's water supply to a newly developed residential area. The contract is structured as a **Unit Price** agreement, with pricing based on linear feet of installed water pipe. However, the project involves various complexities, including excavation in diverse soil conditions, installation of specialized pipe fittings, and coordination with multiple utility companies.

During the initial phase, discrepancies arose regarding the interpretation of unit prices. The client questioned whether the linear foot price included all necessary components, such as excavation, backfill, and pipe fittings. Additionally, unexpected soil conditions required the use of specialized excavation techniques, leading to increased labor costs. Furthermore, permit delays resulted in the need for overtime work to meet the project deadline.

Based on the 'Waterline Expansion Project' case study, thoroughly investigate and document the advantages and disadvantages of employing a **Unit Price** contract for this type of civil engineering project.

[10]

[03+07]

### University of Asia Pacific Department of Civil Engineering Mid-Term Examination, Fall 2024

Course # CE 441	Course Title: Geotechnical Engineering II
Full Marks: 40	Time: 1 hour

#### Answer to all questions

- 1. (a) Mention three major purposes of geotechnical sub-surface exploration. Also mention the 4 names of any four in-situ tests.
  - (b) Mention the preliminary information that should be available to conduct a subsurface 3 exploration program for (i) a building project, and (ii) a bridge project.
  - (c) Summarize your understanding regarding disturbed and undisturbed sampling in terms of 6 disturbances.
- 2. Consider the following scenario for an existing three-storied building:
  - An existing footing is found to have dimension of 8 ft x 8 ft
  - Estimated column load on this footing for existing condition,  $P_{app} = 400$  kips
  - Analyzed ultimate bearing capacity as determined from geotechnical analysis performed for this site,  $(q_u) = 12,500$  psf.

Assess the actual factor of safety of the existing footing and comment on your result. Also estimate the revised size of the footing required to maintain required factor of safety of 2.5

3. Information regarding a geotechnical site investigation conducted using a hammer with efficiency of 55% at a site is provided below. Estimate the Field SPT values for all depths. Apply necessary corrections (for 60% energy and overburden) and calculate the corrected SPT values as required (Use Appendix, as necessary) at depths of *10 feet and 15 feet* below EGL. Also determine the angle of internal friction and cohesion, as applicable at corresponding depths. Consider use of liner.

DryZ	one		
- 8.0 ft	/1/2 Mois	↑ t Zone	<u> </u>
- 20.0 ft — • Blow Counts	Saturated 3/5/7 Non-Plastic SILT	$\begin{array}{l} \gamma = 100 \; \mathrm{Pcf} \\ \gamma_{\mathrm{d}} = 90 \; \mathrm{Pcf} \\ \gamma_{\mathrm{sat}} = 122.4 \; \mathrm{Pc} \end{array}$	cf
- 26.0 ft • Blow Counts 2	/3/3 Saturated /3/3 Plastic SILT	$\gamma = 110 \text{ Pcf}$ $\gamma_d = 95 \text{ Pcf}$	$\gamma_{sat} = 117.4 \text{ Pcf}$

4. Using Terzaghi's Bearing Capacity Equation (TBCE), as appropriate, determine the size of 12 the square footing for long term (drained) condition as shown in the figure below. Also calculate the allowable bearing capacity in this case.



8

7

Course Title: Structural Engineering V		Course Code: CE 415
Time: 1 hour	Credit Hour: 2.00	Full Marks: 60

#### Answer all the questions

### **QUESTION 1 [20 MARKS]**

A prestressed concrete rectangular beam 500 mm by 800 mm has a simple span of 6.5 m and is loaded by a uniform load of 45 kN/m including its own weight, as shown in **Figure 1**. The prestressing tendon is parabolic and produces an effective prestress of 1620 kN. Compute the extreme fiber stresses in the concrete at the midspan section using the **Flexural Stress Method** (1<sup>st</sup> Concept).

[15]



Figure 1: Simply Supported beam with parabolic tendon

### **QUESTION 2 [10 MARKS]**

Describe briefly about the following topics:

- i. Nature of Concrete-Steel Interface
- ii. Stages of Loading
- iii. Advantages of Prestressed Concrete

[10]

### **QUESTION 3 [15 MARKS]**

a. A straight pretensioned concrete member of 15 m long, with a cross section of 400 mm by 400 mm, is concentrically prestressed with 800 mm<sup>2</sup> of steel wires which are anchored to the bulkheads with a stress of 1050 MPa. If  $E_c = 35000$  MPa and  $E_s = 200000$  MPa, compare (i) the **Exact loss** of prestress, and (ii) the **Approximate loss** of prestress due to the elastic shortening of concrete at the transfer of prestress.

[8]

b. A straight cable 20 m long is to be tensioned from one end to an initial prestress of 1035 MPa immediately after transfer. The shrinkage of concrete is 0.0005 and the average compression of concrete is 6 MPa per unit length of the tendon. If  $E_c = 30000$  MPa and  $E_s = 200000$  MPa, determine the **length of the shims** required, neglecting elastic shortening and friction of tendon.

[7]

### **QUESTION 4 [15 MARKS]**

A posttensioned bonded concrete beam, as shown in **Figure 2**, has a prestress of 1580 kN in the steel immediately after prestressing, which eventually reduces to 1300 kN due to losses. The beam carries three live loads of 50 kN each in addition with its self-weight of 5 kN/m. The cross section of the midspan, as shown in **Figure 3**, has 500 mm width and 800 mm depth. Determine the extreme fiber stresses in midspan under the **Final condition** after the losses have been taken place, and with full live load using the **Internal Couple Resisting Method** ( $2^{nd}$  Concept).

[20]



Figure 2: Beam Elevation

Figure 3: Midspan Section

# University of Asia Pacific Department of Civil Engineering Midterm Examination Fall 2024 Program: B.Sc. Engineering (Civil)

Course Title: Structural Engineering III		Course Code: CE 411
Time: 1 hour	Credit Hour: 3.0	Full Marks: $40 (= 4 \times 10)$

ANSWER ALL QUESTIONS. Any missing data can be assumed reasonably.

- 1. Use stiffness method to calculate rotations of joint *d* of the grid system *abcdef* loaded as shown in <u>*Fig.1*</u> [Given:  $EI = 50 \times 10^3$  k-ft<sup>2</sup> and  $GJ = 5 \times 10^3$  k-ft<sup>2</sup>].
- Ignore zero-force members of the space truss *abcdefgh* shown in *Fig.2* and apply boundary conditions to formulate stiffness matrix and load vector.
  [Given: S<sub>x</sub> = 1000 k/ft, Nodal Coordinates (*ft*) are a(0,12,0), b(0,12,-5), c(6,12,-5), d(6,12,0), e(0,0,0), f(0,0,-5), g(6,0,-5) and h(6,0,0)].



- Identify zero-force members of the truss loaded as shown in *Fig.3*. Determine the displacements of joints *a* and *b*.
   [Given: EA/L =1000 k/ft].
- Use stiffness method (neglecting axial deformations) to calculate deflection of joint *f* and rotation of joint *d* of the frame *abcdef* loaded as shown in *Fig.4* [Given: EI = 50×10<sup>3</sup> k-ft<sup>2</sup>]

#### List of Useful Formulae for CE 411

\* The stiffness matrix  $K_m^G$  of a 2D truss member in the global axis system is given by

$$\mathbf{K}_{m}{}^{G} = S_{x} \begin{pmatrix} C^{2} & CS & -C^{2} & -CS \\ CS & S^{2} & -CS & -S^{2} \\ -C^{2} & -CS & C^{2} & CS \\ -CS & -S^{2} & CS & S^{2} \end{pmatrix} \text{ and Truss member force, } P_{AB} = S_{x} [(u_{B}-u_{A}) C + (v_{B}-v_{A}) S] \\ \text{ [where } C = \cos\theta, S = \sin\theta]$$

Fixed End Reactions for One-dimensional Prismatic Members under Typical Loadings



\* The stiffness matrix of a 3D truss member in the global axes system [using  $C_x = \cos \alpha$ ,  $C_y = \cos \beta$ ,  $C_z = \cos \gamma$ ] is

$$\mathbf{K}_{m}{}^{G} = S_{x} \begin{pmatrix} C_{x}^{2} & C_{x}C_{y} & C_{x}C_{z} & -C_{x}^{2} & -C_{x}C_{y} & -C_{x}C_{z} \\ C_{y}C_{x} & C_{y}^{2} & C_{y}C_{z} & -C_{y}C_{x} & -C_{y}^{2} & -C_{y}C_{z} \\ C_{z}C_{x} & C_{z}C_{y} & C_{z}^{2} & -C_{z}C_{x} & -C_{z}C_{y} & -C_{z}^{2} \\ -C_{x}^{2} & -C_{x}C_{y} & -C_{x}C_{z} & C_{x}^{2} & C_{x}C_{y} & C_{x}C_{z} \\ -C_{y}C_{x} & -C_{y}^{2} & -C_{y}C_{z} & C_{y}C_{x} & C_{y}^{2} & C_{y}C_{z} \\ -C_{z}C_{x} & -C_{z}C_{y} & -C_{z}^{2} & C_{z}C_{x} & C_{z}C_{y} & C_{z}^{2} \end{pmatrix}$$

$$rce P_{AB} = S_{x} \left[ (\mu_{B} - \mu_{A}) C_{x} + (\nu_{B} - \nu_{A}) C_{y} + (w_{B} - w_{A}) C_{z} \right]$$

\* Member force  $P_{AB} = S_x [(u_B - u_A) C_x + (v_B - v_A) C_y + (w_B - w_A) C_z]$ 

\* Torsional stiffness  $T_1 = GJ/L$ 

\* Ignoring axial deformations, the matrices  $K_m^L$  and  $G_m^L$  of a frame member in the local axis system are

$$\mathbf{K_m^L} = \begin{pmatrix} S_1 & S_2 & -S_1 & S_2 \\ S_2 & S_3 & -S_2 & S_4 \\ -S_1 & -S_2 & S_1 & -S_2 \\ S_2 & S_4 & -S_2 & S_3 \end{pmatrix} \qquad \qquad \mathbf{G_m^L} = (P/30L) \begin{pmatrix} 36 & 3L & -36 & 3L \\ 3L & 4L^2 & -3L & -L^2 \\ -36 & -3L & 36 & -3L \\ 3L & -L^2 & -3L & 4L^2 \end{pmatrix}$$

where  $S_1 = 12EI/L^3$ ,  $S_2 = 6EI/L^2$ ,  $S_3 = 4EI/L$ ,  $S_4 = 2EI/L$ 

\*  $\mathbf{K}_{\text{total}} = \mathbf{K} + \mathbf{G}$ , buckling occurs (i.e.,  $P = P_{\text{cr}}$ ) when  $|\mathbf{K}_{\text{total}}| = 0$ 

\* For sections of Elastic-Fully-Plastic material,  $A_t = A_c = A/2$ , and  $M_p = A_c \overline{y_c} + A_t \overline{y_t}$ 

- \* For RC sections,  $M_p = A_s f_y (d-a/2)$ , where  $a = A_s f_y/(0.85 f_c' b)$
- \* Virtual work done by external forces ( $\delta W_E$ ) = Virtual work done by internal forces ( $\delta W_I$ )
- \* For simply supported beams under (i) concentrated midspan load  $P_u = 4 M_p/L$ , and (ii) UDL  $w_u = 8 M_p/L^2$
- \* For fixed-ended beams under (i) concentrated midspan load  $P_u = 8 M_p/L$ , and (ii) UDL  $w_u = 16 M_p/L^2$
- \* For hinged-fixed ended beams under UDL  $w_u = 11.66 M_p/L^2$
- \* Using CAA Method,  $(m + c\Delta t/2 + k\Delta t^2/4)a_{i+1} = f_{i+1} ku_i (c + k\Delta t)v_i (c\Delta t/2 + k\Delta t^2/4)a_i$ [m = Total mass, c = Damping =  $2\xi\sqrt{(km)}$ , where  $\xi$  = Damping Ratio] Also  $v_{i+1} = v_i + (a_i + a_{i+1})\Delta t/2$ , and  $u_{i+1} = u_i + v_i \Delta t + (a_i + a_{i+1})\Delta t^2/4$ , starting with  $a_0 = (f_0 - cv_0 - ku_0)/m$

\* Lumped- and Consistent-Mass matrix for axial rod [ Consistent-Mass matrix for beam [ $\mu$  = Mass per unit length]

0

 $M_{\rm m} = (\mu L/2) [1]$ 

	~			~	-
	156	22L	54	-13L )	
$M_{\rm m} = (\mu L/420)$	22L	$4L^2$	13L	-3L <sup>2</sup>	
	54	13L	156	-22L	
	-13L	-3L <sup>2</sup>	-22L	$4L^2$	

\* At natural frequency (i.e.,  $\omega = \omega_n$ ),  $|\mathbf{K} - \omega_n^2 \mathbf{M}| = 0$ 

\* Stiffness of Circular Surface Foundations on Half-Space

 $\mathbf{M}_{\mathbf{m}} = (\mu L/3) \begin{pmatrix} 1 & 0.5 \\ 0.5 & 1 \end{pmatrix}$ 

Motion	Horizontal	Vertical	Rotational	Torsional
K <sub>Halfspace</sub>	$8G_{s}R/(2-\nu)$	$4G_{s}R/(1-v)$	$8G_{s}R^{3}/(3-3v)$	16G <sub>s</sub> R <sup>3</sup> /3